Chapter 15

Preapproved Wall Appendix: Specific Requirements and Details for Tensar ARES Walls

In addition to the general design requirements provided in **Appendix 15-A**, the following specific requirements apply:

The detailed design methodology, design properties, and assumptions used by Tensar Earth Technologies for the ARES wall are summarized in the HITEC evaluation report for this wall system (HITEC, 1997, *Evaluation of the Tensar ARES Retaining Wall System*, ASCE, CERF Report No. 40301). The design methodology, which is based on the Standard Specifications for Highway Bridges (2002) is consistent with the general design requirements in **Appendix 15-A**, except as noted below. Interim approval is given for the continued use of the AASHTO Standard Specifications as the basis for design.

Reinforcement pullout shall be calculated based on the default values for geogrid reinforcement provided in the AASHTO Specifications. For LRFD based design, while it is recognized that product and soil type specific pullout interaction coefficients obtained in accordance with the AASHTO LRFD Specifications for the Tensar products used with this wall system are provided in the HITEC report for the ARES Wall system, pullout resistance design using these product and soil specific interaction coefficients has not been calibrated using the available product specific data statistics and reliability theory. Therefore, the specified resistance factors in the GDM and AASHTO LRFD Specifications should not be considered applicable to the product specific pullout interaction coefficients provided in the HITEC report.

The reinforcement long-term tensile strengths (T_{al}) provided in the WSDOT Qualified Products List (QPL) for the Tensar Geogrid product series, which are based on the 2003 version of the product series, shall be used for wall design, until such time that they are updated, and the updated strengths approved for WSDOT use in accordance with WSDOT Standard Practice T925. Until such time that the long-term reinforcement strengths are updated, it shall be verified that any material sent to the project site for this wall system is the 2003 version of the product. Furthermore, the short-term ultimate tensile strengths (ASTM D6637) listed in the QPL shall be used as the basis for quality assurance testing and acceptance of the product as shipped to the project site per the WSDOT Standard Specifications for Construction.

The HITEC report provided details and design criteria for a panel slot connector to attach the geogrid reinforcement to the facing panel. Due to problems with cracking of the facing panel at the location of the slot, that connection system has been discontinued and replaced with a full thickness panel in which geogrid tabs have been embedded into the panel. For this new connection system, the geogrid reinforcement is connected to the geogrid tab through the use of a Bodkin joint. Construction and fabrication inspectors should verify that the panels to be used for WSDOT projects do not contain the discontinued slot connector.

The Bodkin connection test results provided by letter to WSDOT dated September 28, 2004, were performed on the 2003 version of the Tensar geogrid product line. In that letter, it was stated that UMESA6 (UX1700HS) will typically be used for the connector tabs, regardless of the product selected for the reinforcement. If a lighter weight product is used for the connector tabs, the connection strength will need to be reduced accordingly. **Table 15-(Tensar ARES)-1** provides a summary of the connection strengths that are approved for use with the ARES wall system.

Tensar Soil Reinforcement Geogrid Product	Tensar Panel Connector Tab Geogrid Product	T _{ult} (MARV) for Geogrid Reinforcement per ASTM D6637 in WSDOT QPL (lbs/ft)	CRu	RF	T _{ac} (lbs/ft)
UMESA3/ UX1400HS	UMESA6/ UX1700HS	4,820	1.0	3.6	1,340
UMESA4/ UX1500HS	UMESA6/ UX1700HS	7,880	1.0	3.5	2,250
UMESA5/ UX1600HS	UMESA6/ UX1700HS	9,870	1.0	3.4	2,900
UMESA6/ UX1700HS	UMESA6/ UX1700HS	12,200	0.91	3.3	3,360
UMESA3/ UX1400HS	UMESA3/ UX1400HS	4,820	0.85	3.6	1,140
UMESA4/ UX1500HS	UMESA4/ UX1500HS	7,880	0.79	3.5	1,780
UMESA5/ UX1600HS	UMESA5/ UX1600HS	9,870	0.87	3.4	2,530
UMESA6/ UX1700HS	UMESA6/ UX1700HS	12,200	0.91	3.3	3,360

Table 15-(Tensar ARES)-1 Approved connection strength design values for Tensar ARES walls.

Tac, the long-term connection strength, shall be calculated as follows for the Tensar ARES wall:

$$T_{ac} = \frac{T_{MARV} \bullet CR_u}{RF}$$
 (15-(Tensar ARES)-1)

where,

$$RF = RF_{ID} \times RF_{CR} \times RF_{D}$$

and,

 T_{MARV} = the minimum average roll value for the ultimate geosynthetic strength T_{ult} ,

 $CR_u =$ the ultimate connection strength $T_{ultconn}$ divided by the lot specific ultimate tensile

strength, T_{lot} (i.e., the lot of material specific to the connection testing),

RF_{ID} = reduction factor for installation damage,

 RF_{CR} = creep reduction factor for the geosynthetic, and RF_{D} = the durability reduction factor for the geosynthetic.

Approved details for the Tensar ARES wall system are provided in the following plan sheets. Exceptions and additional requirements regarding these approved details are as follows:

- For all plan sheets, the full height panel details are not preapproved. Full height panels may be used by special design, with the approval of the State Bridge Design Engineer and the State Geotechnical Engineer, provided a complete wall design with detailed plans are developed and included in the construction contract (i.e., full height panel walls shall not be submitted as shop drawings in design-bid-build projects).
- In plan sheet 3 of 19, there should be a minimum cover of 4 inches of soil between the geogrid and the traffic barrier reaction slab.
- In plan sheet 8 of 19, the strength of the geogrid and connection available shall be reduced by 10% to account for the skew of the geogrid reinforcement. The skew angle relative to the perpendicular from the wall face shall be no more than 10°.
- In plan sheets 10 and 14 of 19, regarding the filter fabric shown, WSDOT reserves the right to require the use WSDOT Standard Specification materials as specified in Standard Specification Section 9-33 that are similar to those specified in this plan sheet.
- In plan sheet 15 of 19, the guard rail detail, the guard rail post shall either be installed through precut holes in the geogrid layers that must penetrated, or the geogrid layers shall be cut in a manner that prevents ripping or tearing of the geogrid.

































